| TABLE OF REINFORCING STEEL DIMENSIONS AND DATA |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DESIGN H | $6^{\prime}$ | 8 | $10^{\prime}$ | $12^{\prime}$ | $14^{\prime}$ | $16^{\prime}$ | $18^{\prime}$ | $20^{\prime}$ | 22' | $24^{\prime}$ | $26^{\prime}$ | $28^{\prime}$ | $30^{\prime}$ | $32^{\prime}$ |
| w | $6^{\prime}-9^{\prime \prime}$ | $7^{\prime}$ - $0^{\prime \prime}$ | $7^{\prime}-6{ }^{\prime \prime}$ | 7'-9" | $8^{\prime}$-3" | 9'0" | $10^{\prime}-0^{\prime \prime}$ | 11'-0" | $12^{\prime}-3^{\prime \prime}$ | $13^{\prime}-3^{\prime \prime}$ | $14^{\prime}-3^{\prime \prime}$ | $15^{\prime}-3^{\prime \prime}$ | $16^{\prime}-6^{\prime \prime}$ | 17'-6" |
| c | $2^{\prime}-0^{\prime \prime}$ | $2^{\prime}-2^{\prime \prime}$ | $2^{\prime}-4^{\prime \prime}$ | $2^{\prime}-3^{\prime \prime}$ | $2^{\prime}-6^{\prime \prime}$ | 2'-9" | $2^{\prime}-10^{\prime \prime}$ | 3'-8" | $4^{\prime \prime} 1^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | $4^{\prime}-10^{\prime \prime}$ | $5^{\prime}-2^{\prime \prime}$ | $5^{\prime}-7^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ |
| B | $4^{\prime}-9{ }^{\prime \prime}$ | $4^{\prime}-10^{\prime \prime}$ | $5^{\prime}-2^{\prime \prime}$ | $5^{\prime \prime-}{ }^{\prime \prime}$ | $5^{\prime \prime-91}$ | $6^{\prime}-3^{\prime \prime}$ | $7^{\prime \prime}-2^{\prime \prime}$ | $7^{\prime}-4^{\prime \prime}$ | $8^{\prime \prime}-2^{\prime \prime}$ | $8^{\prime}-9^{\prime \prime}$ | $9^{\prime \prime}-5^{\prime \prime}$ | $10^{\prime}-1^{\prime \prime}$ | $10^{\prime}-11^{\prime \prime}$ | 11'-6" |
| F PILE FOOTING | $1^{\prime \prime}-9^{\prime \prime}$ | $1^{\prime}-9^{\prime \prime}$ | $1^{\prime \prime}$ '9" | 1'-9" | $1^{\prime \prime}-9$ " | 2'-0" | $2^{\prime}-0^{\prime \prime}$ | 2'-6" | $2^{\prime \prime}-9^{\prime \prime}$ | $2^{\prime}-9{ }^{\prime \prime}$ | 3'-0" | 3'-3" | $3^{\prime}-9$ " | $4^{\prime}-0^{\prime \prime}$ |
| M | $0^{\prime}-6^{\prime \prime}$ | $0^{\prime}-8^{\prime \prime}$ | $0^{\prime}-10^{\prime \prime}$ | $0^{\prime}-9^{\prime \prime}$ | $1^{\prime}$ - $0^{\prime \prime}$ | $1^{\prime}-3^{\prime \prime}$ | $1^{\prime \prime}-4^{\prime \prime}$ | 2'-2' | $2^{\prime}-7^{\prime \prime}$ | $3^{\prime}-0^{\prime \prime}$ | 3'-4" | $3^{\prime}-8^{\prime \prime}$ | $4^{\prime}-1^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ |
| N | $3^{\prime}-3^{\prime \prime}$ | $3^{\prime \prime}-4^{\prime \prime}$ | $3^{\prime}-8^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-3^{\prime \prime}$ | 4'-9" | $5^{\prime}-8^{\prime \prime}$ | $5^{\prime \prime}-10^{\prime \prime}$ | $6^{\prime}-8^{\prime \prime}$ | $7^{\prime}-3^{\prime \prime}$ | $7^{\prime \prime-11^{\prime \prime}}$ | $8^{\prime}-7{ }^{\prime \prime}$ | $9^{\prime \prime}-5^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ |
| Row 1 SPACING | $12^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ | $8^{\prime}-9^{\prime \prime}$ | $7^{\prime}-3^{\prime \prime}$ | $6^{\prime}-3^{\prime \prime}$ | $5^{\prime}-3^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | 5'-3" | $4^{\prime}-9^{\prime \prime}$ | $4^{\prime \prime}-6^{\prime \prime}$ | $4^{\prime}$ - $0^{\prime \prime}$ | 3'-9" | 3'-9" | $3^{\prime}-6^{\prime \prime}$ |
| ROW 2 SPACING | $13^{\prime}-3^{\prime \prime}$ | $13^{\prime}-3^{\prime \prime}$ | $12^{\prime}-3^{\prime \prime}$ | $10^{\prime}-3^{\prime \prime}$ | $9^{\prime}-6^{\prime \prime}$ | $8^{\prime}-3^{\prime \prime}$ | $7^{\prime}-3^{\prime \prime}$ | $6^{\prime}-9^{\prime \prime}$ | $6^{\prime \prime}-6^{\prime \prime}$ | $5^{\prime}-9^{\prime \prime}$ | $5^{\prime}-0^{\prime \prime}$ | $4^{\prime}-3^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ | $4^{\prime}-0^{\prime \prime}$ |
| ROW 3 SPACING | - | - | - | - | - | - |  | $7^{\prime}-6^{\prime \prime}$ | $6^{\prime}-9^{\prime \prime}$ | $6^{\prime}-0^{\prime \prime}$ | $5^{\prime}$-6" | $5^{\prime}-0^{\prime \prime}$ | $4^{\prime}-6^{\prime \prime}$ | 4'-3" |
| ROW 4 SPACING | - | - | - | - | - | - | - | - | - | - | - | - | $5^{\prime}-0^{\prime \prime}$ | $4^{\prime}-9^{\prime \prime}$ |
| BATTER | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 58:12 | 5/8:12 | 5/8:12 | 3/4:12 | 3/4":12 | 7/8:12 | 1:12 | 1:12 |
| (0) BARS | - | - | - | \#5 ¢ 6 | \#5 ¢ 6 | \#5 ¢ 6 | \#5 @ 6 | \#5 @ 6 | \#6 @ 8 | \#6 @ 8 | \#6 @ 8 | \#6 @ 8 | \#6 ¢ 8 | \#6 ¢ 8 |
| (D) BARS | \#5 @ 6 | \#5 @ 5 | \#7 @ 7 | \#7 @ 6 | \#8 @ 6 | \#8 @ 6 | \#9 @ 6 | \#9 @ 6 | \#8 ¢ 88 | \#9 @ 88 | \#9 @ 88 | \#9 @ 88 | \#10 ¢ 88 | \#10@ 88 |
| ha | - | - | - | - | - | - | - | - | 12'-0' | $12^{\prime}-0^{\prime \prime}$ | $14^{\prime}-0^{\prime \prime}$ | $14^{\prime}-6^{\prime \prime}$ | 14'-0" | $16^{\prime}-0{ }^{\prime \prime}$ |
| nb | - | - | - | $6^{\prime}-0^{\prime \prime}$ | $8^{\prime}-0^{\prime \prime}$ | $10^{\prime}-0^{\prime \prime}$ | $11^{\prime}-0{ }^{\prime \prime}$ | $13^{\prime}-0^{\prime \prime}$ | $16^{\prime}-6^{\prime \prime}$ | $18^{\prime}-6^{\prime \prime}$ | 20'-0' | 20'-6" | $22^{\prime}-0^{\prime \prime}$ | $24^{\prime}-0^{\prime \prime}$ |
| © BARS | \#5 @ 12 | \#5 @ 10 | \#5 ¢ 7 | \#5 ¢ 6 | \#5 @ 6 | \#5 ¢ 6 | \#6 @ 6 | \#7 © 6 | \#8 @ 8 | \#8 @ 8 | \#9 @ 8 | \#6 © 4 | \#9 ¢ 8 | \#9 @ 8 |
| (0) BARS | \#5 @ 12 | \#5 @ 10 | \#5 @ 7 | \#5 @ 6 | \#5 @ 6 | \#5 © 6 | \#7 @ 6 | \#5 @ 6 | \#5 @ 8 | \#6 @ 8 | \#7 @ 8 | \#5 @ 4 | \#6 © 8 | \#7 @ 8 |
| (e) BARS | \#6 @ 4 | \#6 ¢ 4 | \#5 @ 4 | \#5 @ 4 | \#5 @ 5 | \#5 @ 8 | \#5 @ 10 | \#5 ¢ 9 | \#5 @ 10 | \#5 @ 10 | \#6 @ 12 | \#6 @ 12 | \#6 @ 12 | \#6 @ 12 |
| (f) BARS | \#7 @ 4 | \#7 @ 4 | \#6 @ 4 | \#6 @ 4 | \#6 @ 5 | \#6 ¢ 7 | \#5 @ 7 | \#5 ¢ 7 | \#5 @ 8 | \#5 @ 9 | \#6 @ 12 | \#6 ¢ 12 | \#6 @ 12 | \#6 @ 12 |

8: 2 bar bundle

"ha" and "hb" above (B) bars indicate distance from top
of foot ing to upper end of (1) bars, see


## NOTES:

1. All piles are class 90 concrete piles.
2. Pile batter shown are 1:3.
3. Minimum distance between center of pile and edge of
footing is $1^{\prime}-6^{\prime \prime}$.
4. Lateral resistance of each pile:
18 kip for service limit states

30 kip for strength imit states.
40 kip for extreme event limit states
5. Soil passive resistance with $\phi=34^{\circ}$ considered for strength and extreme event limit states.
Soil friction on footing bottom ignored.
6. Maximum spacing between piles is shown in the table.
Reduce to suit the length of footing.
7. Minimum distance between any two piles is $3^{\prime}-3^{\prime \prime}$.
8. For sound wall and retaining wall Architectural Treatment,
see details elsewhere in Project Plans.
9. For details not shown and drainage notes, see Standard
0. Footing cover, $1^{1}-6$ " minimum.
11. For sound wall and reinforcement details,


## GENERAL NOTES

## LOAD AND RESISTANCE FACTOR DESIGN

Design: $\begin{gathered}\text { AASHTO LRFD Bridge Design Specifications, } \\ \text { 8th edition with California Amendments, }\end{gathered}$
w: wind perpendicular to plane of sound barrie
Exposure Category D.
LS: Variable live load surcharge on level ground
DC: $\quad \begin{aligned} & \text { Stem Architectural treatment of thickness } \\ & \text { Up to } 2 \text { " of concrete }\end{aligned}$
Seismic: Kn $=0.3$
soil: $\quad \begin{aligned} & \varnothing=34^{\circ} \\ & y=120 \mathrm{pcf}\end{aligned}$
Reinforced ${ }^{\prime}=3600$ psi
Concrete:
fy
$=60,000$
Load Combinations and Limit States
Service I $Q=1.000 \mathrm{C}+1.00 \mathrm{EV}+1.00 \mathrm{EH}+1.00 \mathrm{LS}+1.00 \mathrm{~W}$ strength I $\mathrm{Q}=a \mathrm{AC}+\mathrm{BEV}+\mathrm{nEH}+1.75 \mathrm{LS}$
Strength III $\mathrm{Q}=\mathrm{ODC}+\mathrm{BEV}+1.50 \mathrm{EH}+1.00 \mathrm{WS}$
strength $V \quad Q=a D C+B E V+1.50 E H+1.35 L S+1.00 W S$
Extreme I $Q=1.00 \mathrm{DC}+1.00 \mathrm{EV}+1.00 \mathrm{EH}+1.00 \mathrm{EOD}+1.00 \mathrm{EOE}$ where:

Q: Force Effects
1.25 or 0.90 , whichever Controls Design
a:
1.35 or 1.00 , Whichever Controls Desigh D: 0.9 or 1.5, whichever Controls Design DC: Dead Load of Structural C
EH: Horizontal Earth Pressure
EV: Vertical Earth Fi川Pressure
LS: Live Load Surcharge
EQE: Seismic Earth Pressure
EQD: Soil and Structural Components Inertio
ws. Soil inertia ignored for stem design

