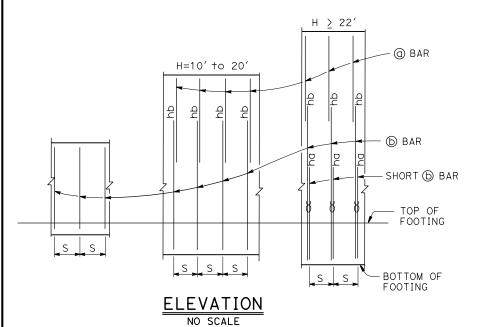
TABLE OF REINFORCING STEEL DIMENSIONS AND DATA														
DESIGN H	6′	8′	10′	12′	14′	16′	18′	20′	22′	24′	26′	28′	30′	32′
W	7′-6"	7′-6"	7′-6"	7′-9"	8'-3"	9'-0"	10'-0"	11'-0"	12'-3"	13′-3"	14'-3"	15′-3"	16'-6"	17′-6"
С	2'-0"	2'-0"	2'-4"	2'-3"	2'-6"	2'-9"	2'-10"	3'-8"	4'-1"	4'-6"	4'-10"	5'-2"	5'-7"	6'-0"
В	5'-6"	5'-6"	5'-2"	5′-6"	5′-9"	6'-3"	7'-2"	7'-4"	8'-2"	8'-9"	9'-5"	10'-1"	10'-11"	11'-6"
F PILE FOOTING	1'-9"	1'-9"	1'-9"	1'-9"	1'-9"	2'-0"	2'-0"	2'-6"	2'-9"	2'-9"	3'-0"	3'-3"	3'-9"	4'-0"
M	0'-6"	0'-6"	0'-10"	0'-9"	1 '-0"	1 '-3"	1'-4"	2'-2"	2'-7"	3'-0"	3'-4"	3'-8"	4'-1"	4'-6"
N	4'-0"	4'-0"	3'-8"	4'-0"	4'-3"	4'-9"	5'-8"	5'-10"	6'-8"	7'-3"	7'-11"	8'-7"	9'-5"	10'-0"
ROW 1 SPACING	12'-0"	10'-0"	8'-6"	7'-0"	6'-0"	5'-3"	4'-6"	5'-3"	4'-9"	4'-6"	4'-0"	3'-9"	3'-9"	3'-6"
ROW 2 SPACING	13'-0"	13'-3"	12'-0"	11'-0"	9'-6"	8'-3"	7'-3"	6'-9"	6'-3"	5′-6"	4'-9"	4'-0"	4'-0"	4'-0"
ROW 3 SPACING	-	-	-	-	-	-	_	7′-6"	6'-6"	5′-9"	5'-3"	4'-9"	4'-3"	4'-0"
ROW 4 SPACING	-	-	-	-	-	-	-	-	-	-	-	-	4'-9"	4'-6"
STEM WITH HAUNCH, BATTER	1/2:12	1/2:12	1/2:12	1/2:12	1/2:12	1/2:12	5/8:12	5/8:12	5%:12	3/4":12	3/4":12	7/8:12	1:12	1:12
STEM WITHOUT HAUNCH, BATTER	-	-	-	-	-	-	-	-	1/4:12	1/4:12	1/2:12	3/4:12	3/4:12	3/4:12
@ BARS	-	-	#7 <b>@</b> 8	#7 <b>@</b> 8	#6 @ 6	#6 @ 6	#6 @ 6	#6 @ 6	#7 @ 8	#7 @ 8	#7 @ 8	#7 @ 8	#7 @ 8	#7 @ 8
D BARS	#6 @ 6	#6 @ 6	#8 @ 8	#9 @ 8	#8 @ 6	#8 @ 6	#9 @ 6	#9 @ 6	#8 @ 8 8	#9 @ 8 K	#9 @ 8 8	#9 @ 88	#10 @ 8 g	#10 @ 8 g
ha	-	-	-	-	-	-	-	_	12'-0"	12'-0"	14'-0"	14'-6"	14'-0"	16'-0"
hb	-	-	4'-0"	5'-0"	6'-0"	8'-0"	10'-0"	12'-0"	15'-0"	15'-0"	17'-0"	18'-6"	19'-6"	21'-0"
© BARS	#5 @ 6	#5 <b>@</b> 6	#5 <b>@</b> 8	#5 <b>@</b> 8	#5 <b>@</b> 6	#5 @ 6	#6 @ 6	#7 @ 6	#5 @ 4	#8 @ 8	#6 @ 4	#6 @ 4	#9 @ 8	#9 @ 8
d BARS	#5 @ 6	#5 <b>@</b> 8	#5 <b>@</b> 8	#5 @ 8	#5 @ 6	#5 <b>@</b> 6	#9 @ 12	#6 @ 12	#5 @ 8	#6 @ 8	#7 @ 8	#5 @ 4	#6 @ 8	#7 @ 8
BARS	#6 @ 4	#6 @ 4	#5 @ 4	#5 <b>@</b> 5	#5 @ 5	#5 @ 8	#5 @ 10	#5 @ 8	#5 @ 10	#5 <b>@</b> 10	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12
① BARS	#7 @ 4	#7 <b>@</b> 4	#6 @ 4	#5 @ 4	#6 @ 5	#6 @ 7	#5 @ 7	#5 @ 7	#5 @ 9	#5 @ 9	#6 @ 12	#6 @ 12	#6 @ 12	#6 @ 12

## NOTES:

- 1. All piles are class 90 concrete piles.
- 2. Pile batter shown are 1:3.
- 3. Minimum distance between center of pile and edge of footing is 1'-6".
- 4. Lateral resistance of each pile:
  18 kip for service limit states.
  30 kip for strength limit state. 40 kip for extreme event limit state.
- 5. Soil passive resistance with Ø=34°considered for strength and extreme event limit states. Soil friction at footing bottom ignored.
- 6. Maximum spacing between piles is shown in the table. Reduce to suit the length of footing.
- 7. Minimum distance between any two piles is 3'-3".
- 8. For sound wall and retaining wall Architectural Treatment, see details elsewhere in Project Plans.
- 9. For details not shown and drainage notes, see Standard Plans B0-3, B3-5 & B3-6.
- 10. Footing cover, 1'-6" minimum.
- 11. For sound wall and barrier reinforcement details, see xs15-130-1 and xs15-130-2.



"ha" and "hb" above (b) bars indicate distance from top of footing to upper end of (b) bars, see table.
"S" is (a) and (b) bar spacing, see table.
(c) : 2 bar bundle

- 12. For H=6' and 8', extend (b) bars into barrier for stem with haunch.
- 13. For H≥10', extend ⓐ bars into barrier for stem with haunch.
- 14. Provide additonal #6 x 18'-0" @ 6 bars over a distance of 8'-0" measured from all expansion joints, begin wall and end wall locations. For Ης14', hook the additional #6 bars into footing.

COUNTY POST MILES SHEET REGISTERED CIVIL ENGINEER DATE

PLANS APPROVAL DATE THE STATE OF CALIFORNIA OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

THE REGISTERED CIVIL ENGINEER FOR THE PROJECT IS RESPONSIBLE FOR THE SELECTION AND PROPER APPLICATION OF THE COMPONENT DESIGN AND ANY MODIFICATIONS SHOWN

CIVIL

## GENERAL NOTES LOAD AND RESISTANCE FACTOR DESIGN

Design: AASHTO LRFD Bridge Design Specifications 8th edition with California Amendments, Preface dated April 2019.

Wind perpendicular to plane of sound barrier. Exposure Category D.

Variable live load surcharge on level ground surface

DC: Stem Architectural Treatment of thickness up to 2" of concrete

54 kip transverse force on soundwall applied at 6'-0" above finished grade, distributed over 3'-6" and 1:1 distribution down and outward. Load distribution of 1V:0.6H applied at begin wall, end wall and on either side of expansion

Seismic:  $K_h = 0.3$  $K_{V} = 0.0$ 

LEGEND:

8:2 bar bundle

Soil:  $\emptyset = 34^{\circ}$  $\gamma = 120 \text{ pcf}$ 

Reinforced

Concrete: f'c = 3600 psify = 60,000 psi

Load Combinations and Limit States

Q=1.00DC+1.00EV+1.00EH+1.00LS+1.00WS Service I

Strength I  $Q=aDC+BEV+\eta EH+1.75LS$ 

Strength III Q=aDC+BEV+1.50EH+1.00WS

Strength V Q=aDC+BEV+1.50EH+1.35LS+1.00WS

Q=1.00DC+1.00EV+1.00EH+1.00EQD+1.00EQE Extreme I

Extreme II Q=1.00DC+1.00EV+1.00EH+1.00CT

## Where:

Q: Force Effects

a: 1.25 or 0.90, Which ever Controls Design

B: 1.35 or 1.00, which ever Controls Design

DC: Dead Load of Structural Components

EV: Vertical Earth Fill Pressure

LS: Live Load Surcharge

EQE: Seismic Earth Pressure

EQD: Soil and Structural Components Inertia Soil inertia ignored for stem design

WS: Wind Load on Sound Wall and Barrier

CT: Vehicular Collision Force

BRIDGE STANDARD DETAILS		D DETAILS		STATE OF		BRIDGE No.		v		
<b>X\$14-320-1</b> FILE NO.	April 2022	The components of the Bridge Standard Details have been prepared under the responsible charge of the Technical Owner, a registered civil engineer in the State of California		CALIFORNIA DEPARTMENT OF TRANSPORTATION	DIVISION OF ENGINEERING SERVICES	XX-XXXX POST MILE X.X	RETAININ	G WALL TYPE	1SWBP-DET	AILS No.1
	ot.ca.gov/hq/esc/techpub: tandard-detail-sheets/in		DATE PLOTTED => 8-MAR-2022 TIME PLOTTED => 10:37 ORIGINAL SCALE  FILE => 20220308 vs.14-320-1 dop		UNIT: XXXX		OUTE: XXX/XXX	DISREGARD PRINTS BEARING EARLIER REVISION DATES	REVISION DATES	SHEET OF