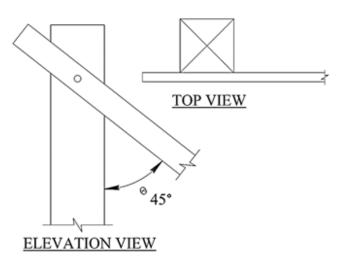


# Appendix D Example 10 – Bolted Joints - Single Shear

Refer to *Falsework Manual,* Section 5-3, *Timber Fasteners*. This example demonstrates how to calculate the capacity of the connection between a diagonal brace and post. For this example, wind load is the governing load.



# **Given Information**

Posts: 12 x 12 Rough Douglas Fir-Larch #1 (G=0.50)

Diagonal Braces: 2 x 8 S4S Douglas Fir-Larch #2 (G=0.50)

Connectors: <sup>3</sup>⁄<sub>4</sub>" Ø Bolt

#### Figure D-10-1. Post and Brace Bolted Joint

Determine the connection capacity between brace and post for Wind Load

Main Member Properties		Side Member Properties	
l <sub>m</sub> = 12 in	thickness (12 x 12)	l <sub>s</sub> = 1.5 in	thickness (2 x 8)
$t_m = I_m = 12$ in		$t_s$ = $I_s$ = 1.5 in	
$\theta_{\rm m}$ = 45°	angle between direction of loading & direction of grain	$\theta_s = 0^\circ$	angle between direction of loading & direction of grain
G = 0.50	Specific Gravity NDS Table 12.3.3		

#### **Connector Properties**

 $\begin{array}{ll} \mathsf{D} = 0.75 \text{ in} & \text{connector diameter} \\ \mathsf{F}_{yb} = 45000 \text{ psi} & Yield Strength (See Footnote #2 NDS Table 12A) \\ \mathsf{F}_{e.pll} = 11200\text{G psi} = 5600 \text{ psi} & Dowel Bearing Strength Parallel to Grain (NDS table 12.3.3 footnote 2) \\ \mathsf{F}_{e.perp} = \frac{6100\text{G}^{1.45}}{\sqrt{\frac{\mathsf{D}}{\mathsf{in}}}} = 2578 \text{ psi} & Dowel Bearing Strength Perpendicular to Grain (NDS table 12.3.3 footnote 2) \end{array}$ 

Compare values to NDS Table 12.3.3:

 $F_{e.pll (NDS Table 12.3.3)} = 5600 \text{ psi}$   $F_{e.perp (NDS Table 12.3.3)} = 2600 \text{ psi}$ Use calculated value for  $F_{perp} = 2578 \text{ psi}$ 

# ulated value for $F_{perp}$ = 2578 psi

### Find Dowel Bearing Strength at an Angle to Grain (NDS Section 12.3.4):

 $F_{em} = \frac{F_{e.pll}F_{perp}}{F_{e.pll}(\sin(\theta_m))^2 + F_{perp}(\cos(\theta_m))^2} = 3531 \text{ psi}$  $F_{es} = \frac{F_{e.pll}F_{perp}}{F_{e.pll}(\sin(\theta_s))^2 + F_{perp}(\cos(\theta_s))^2} = 5600 \text{ psi}$ 

### Find Reduction Term, Rd (NDS Table 12.3.1B):

$\theta = \max(\theta_{m}, \theta_{s}) = 45^{\circ}$	Maximum angle between direction of load and direction of grain for any member in connection (See Table 12.3.1B)
$K_{\theta} = 1 + 0.25 \frac{\theta}{90 \text{ deg}} = 1.125$	
$R_{d_l}$ = 4 $K_{\theta}$ = 4.50	Reduction Term for Yield Mode $I_m$ and $I_s$

$R_{d_{II}} = 3.6 K_{\theta} = 4.05$	Reduction Term for Yield Mode II
R <sub>d</sub> III.IV = 3.2 K <sub>θ</sub> = 3.60	Reduction Term for Yield Mode III <sub>m</sub> , III <sub>s</sub> , and IV

#### Find Yield Limit Equations for Single Shear (NDS Table 12.3.1A):

The controlling value is the minimum single shear capacity from the above equations.

$$Z_{\text{control}} = \min (Z_{\text{Im}}, Z_{\text{Is}}, Z_{\text{II}}, Z_{\text{IIIm}}, Z_{\text{IIIs}}, Z_{\text{IV}}) = 956 \text{ lb}$$
 (Yield Mode IIIs controls)

#### Find Adjusted Lateral Design Value, Z':

#### Adjustment factors from NDS Table 11.3.1:

- C<sub>D</sub> = 1.60 Duration Factor for wind load
- C<sub>M</sub> = 1.0 Wet Service Factor NDS 11.3.3 (Assume < 19% moisture content)
- $C_t = 1.0$  Temperature Factor NDS 11.3.4 (Temp up to 100°F)
- C<sub>g</sub> = 1.0 Group Action Factor NDS 11.3.6 (Single Fastener)
- $C_{\Delta}$  = 1.0 Geometry Factor NDS 12.5.1 (Assume End Dist. & Spacing meet NDS Tables 12.5.1A and 12.5.1B)
- C<sub>eg</sub> = 1.0 End Grain Factor NDS 12.5.2 (Does not apply)
- C<sub>di</sub> = 1.0 Diaphragm Factor NDS 12.5.3 (Does not apply)
- C<sub>tn</sub> = 1.0 Toe Nail Factor NDS 12.5.4 (Does not apply)

## Adjusted lateral design value Z' = $Z(C_D)(C_M)(C_t)(C_g)(C_{\Delta})$ = 1530 lb