

Appendix D Example 7 – Stability of Shoring Towers

Refer to *Falsework (FW) Manual*, Section 6-6, *Tower Stability*. This example demonstrates how to check the stability of shoring towers.

Given Information

Wind load calculated in example 5:



Figure D-7-1. Wind Load on Towers



Figure D-7-2. Tower Reactions

Check Stability

Check Stability of the Unloaded Towers



Figure D-7-3. Unloaded Towers

1. Calculate the resisting moment (RM) before the bridge concrete is placed: Tower weight W = 0.2 $\frac{\text{kip}}{\text{ft}}$ (104 ft) = 20.8 kip

RM per tower = 10 ft (20 k) + 5 ft (20.8 k) = 304^{ft-k}

- 2. Overturning moment (OTM) = 104 ft (7630 lb/1000) = 794^{ft-k}
- 3. Since OTM = 794^{ft-k} > RM = 304^{ft-k}

Cable bracing is required for unloaded condition.

4. Calculate the force in the cables.



Figure D-7-4. Towers and Cable Bracing

5. Check cables:

Efficiency of clip type connectors = 80% (FW Sect. 5-5.04) Factor of safety = 3.0 (FW Sect. 5-5.06)

Ultimate cable load = 69.2 kip (given)

 $\frac{\text{Safe working load} - \frac{\text{Breaki ng streng th x connect or efficie ncy}}{\text{safety factor}} = \frac{69.2 \text{ kip } (0.80)}{3.0} = 18.5 \text{ kip}$

 $18.5 \frac{\text{kip}}{\text{cable}} \ge 2 \text{ cables} = 37.0 \text{ kip}$

34.0 kip \leq 37.0 kip allowable

Check Stability of the Loaded Towers



Figure D-7-5. Loaded Towers

- 1. Calculate the resisting moment after the bridge concrete is placed. RM of two tower units = 2 [5ft (21 kip)+ 10 ft (95 kip)] = $2110^{\text{ft}-\text{k}}$
- 2. OTM of two tower units = $2 \times 794^{\text{ft}-\text{k}} = 1588^{\text{ft}-\text{k}}$
- 3. Since OTM = $1588^{ft-k} \leq RM = 2110^{ft-k}$

Cable bracing is not required for loaded condition