Biofiltration Swale
Design Guidance

September 2012

California Department of Transportation
Division of Environmental Analysis
Storm Water Program
1120 N Street
Sacramento, California
For individuals with sensory disabilities, this document is available in alternate formats upon request. Please call or write to Storm Water Liaison, Caltrans Division of Environmental Analysis, P.O. Box 942874, MS-27, Sacramento, CA 94274-0001. (916) 653-8896 Voice, or dial 711 to use a relay service.
## Table of Contents

1. **INTRODUCTION** ................................................................................................................... 1
   1.1. OVERVIEW ........................................................................................................................... 1
   1.2. BIOFILTRATION SWALES – A BRIEF DESCRIPTION .......................................................... 1

2. **BASIS OF BIOFILTRATION SWALE DESIGN** ....................................................................... 3
   2.1. DESIGN CRITERIA ............................................................................................................... 3
   2.2. RESTRICTIONS .................................................................................................................... 4

3. **GETTING STARTED** ............................................................................................................ 5
   3.1. PRELIMINARY DESIGN PARAMETERS .............................................................................. 5
   3.2. PRELIMINARY CALCULATIONS .......................................................................................... 6
   3.3. OTHER COMMENTS .......................................................................................................... 8

4. **BMP LAYOUT AND DESIGN** .................................................................................................. 11
   4.1. LAYOUT .......................................................................................................................... 11
   4.2. SITE SPECIFIC DESIGN ELEMENTS .................................................................................. 11

5. **PS&E PREPARATION** .......................................................................................................... 13
   5.1. PS&E DRAWINGS .............................................................................................................. 13
   5.2. SPECIFICATIONS ............................................................................................................ 14
   5.3. PROJECT COST ESTIMATES ............................................................................................. 15

6. **REFERENCES** ..................................................................................................................... 17

A **BIOFILTRATION GUIDANCE-VEGETATION** ....................................................................... 18
   A.1 OVERVIEW ......................................................................................................................... 18
   A.2 SOIL TESTING & INVESTIGATION .................................................................................... 18
   A.3 SOIL AND PLANTING AREA PREPARATION ..................................................................... 19
   A.4 Irrigation Strategies .......................................................................................................... 20
   A.5 PLANTING STRATEGIES .................................................................................................. 20
   A.6 RESTRICTIONS FOR PLANT SELECTION ........................................................................ 22
   A.7 DRAINAGE FACILITIES ................................................................................................... 22
   A.8 PLANT ESTABLISHMENT PERIOD (PEP) ........................................................................ 22
   A.9 DEFINITIONS: .................................................................................................................... 23
   A.10 ADDITIONAL RESOURCES ............................................................................................ 23
Table of Contents

LIST OF TABLES

Table 2-1 Biofiltration Swale Design Criteria ................................................................. 3
Table 3-1 Manning’s Equation solved for particular conditions by assuming depth values .... 7

LIST OF FIGURES

Figure 1-1 Schematic of Biofiltration Swale and Strip ...................................................... 2
Figure 1-2 Typical Swale (SR-78/Melrose Drive).............................................................. 2
Figure 3-1 Typical Biofiltration Swale Cross-Section ....................................................... 6

LIST OF ACRONYMS

BMP Best Management Practice
ft feet
HDM Highway Design Manual
HRT Hydraulic Residence Time
HSG Hydrologic Soil Group
in inches
MEP maximum extent practicable
max maximum
min minimum
NPDES National Pollutant Discharge Elimination System
PPDG Project Planning and Design Guide – Storm Water Quality Handbook
PS&E Plans, Specifications and Estimate
RWQCB Regional Water Quality Control Board
sec second
USCS Unified soil classification system
WQF Water Quality Flow
1. Introduction

1.1. Overview

This document provides guidance to Caltrans Project Engineers and Landscape Architects for incorporating Biofiltration Swale Treatment Best Management Practices (BMPs) into projects during the planning and design phases of Caltrans highways and facilities. The primary functions of this document are to:

- Describe the design criteria of Biofiltration Swales ("Bioswales");
- Present detailing standards and siting limitations;
- Present the formulas used to design Bioswales; and
- Review the required elements for implementing Bioswales into PS&E packages.

The Project Engineer and the District Landscape Architect have different perspectives and responsibilities during the design of Biofiltration Swales. In this document, guidance addressed to the Project Engineer is provided in the numbered sections, while guidance to the Landscape Architect is provided in the Appendix (separate attachment); the latter guidance is considered the best available at this time, but the Headquarters Office of Roadside Management will develop and issue additional guidance as needed.

1.2. Biofiltration Swales – A Brief Description

Biofiltration Swales are one of several BMPs for treatment of stormwater runoff from project areas that are anticipated to produce pollutants of concern (e.g., roadways, parking lots, maintenance facilities, etc.). Bioswales are vegetated, typically trapezoidal channels, which receive and convey storm water flows while meeting water quality criteria and other flow criteria. Pollutants are removed by filtration through the vegetation, uptake by plant biomass, sedimentation, adsorption to soil particles, and infiltration through the soil. Pollutant removal capability is related to channel dimensions, longitudinal slope, and type of vegetation. Bioswales are effective at trapping litter, Total Suspended Solids (soil particles), and particulate metals (Caltrans, 2007).

The following list demonstrates some advantages of utilizing a Bioswale as a Treatment Control BMP.

1. When properly implemented, Bioswales are aesthetically pleasing. Due to the presence of its vegetation, the public views Bioswales as a “landscaped roadside” which would make placement more acceptable than other Treatment BMPs using concrete vaults.

2. Bioswales were determined to be an effective Treatment BMP in reducing sediment and heavy metals, as described in the *BMP Retrofit Pilot Program Final Report* (Caltrans, 2004).
3. In that same report Bioswales were determined to be cost effective and, together with Biofiltration Strips, were among the least expensive Treatment BMP per volume of runoff treated.

4. Bioswales also are well suited to being part of a “treatment-train” system of BMPs and should be considered whenever siting other BMPs that could benefit from pretreatment, especially Biostrips, Infiltration Basins, Infiltration Trenches, and Wet Basins.

Schematics of a Biofiltration Swale adjacent to a Biofiltration Strip are shown below.

Figure 1-1 Schematic of Biofiltration Swale and Strip

Figure 1-2 Typical Swale (SR-78/Melrose Drive)
2. Basis of Biofiltration Swale Design

2.1. Design Criteria

To perform as an effective Treatment BMP, the Biofiltration Swale must meet certain design criteria; the primary factors to be incorporated into the design are found in below Table 2-1.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Min. Value</th>
<th>Max. Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Flow Rate (See Note 1)</td>
<td>For water quality treatment: WQF</td>
<td>For roadway drainage (&quot;Design Event&quot;)</td>
</tr>
<tr>
<td></td>
<td>(See Note 1)</td>
<td>(HDM Topic 831)</td>
</tr>
<tr>
<td>Bottom Width (See Note 2)</td>
<td>0 ft, as v-ditch</td>
<td>Consult with District Hydraulics and District Maintenance (See Note 3)</td>
</tr>
<tr>
<td></td>
<td>2 ft, as trapezoid</td>
<td></td>
</tr>
<tr>
<td>Side Slope (sides of the Bioswale, in cross section)</td>
<td>4H:1V</td>
<td>3H:1V with District Maintenance approval</td>
</tr>
<tr>
<td>Longitudinal Slope</td>
<td>0.25%</td>
<td>1% to 2% preferred. 6% maximum. The resulting depth, velocity and HRT must meet the Interrelationship formula</td>
</tr>
<tr>
<td>Hydraulics Residence Time (HRT) at WQF</td>
<td>5 minutes</td>
<td>No maximum</td>
</tr>
<tr>
<td>Length of flow path</td>
<td>As required by minimum HRT</td>
<td>No maximum</td>
</tr>
<tr>
<td>Flow Depth during WQF</td>
<td>No minimum</td>
<td>6 inches (See Note 4)</td>
</tr>
<tr>
<td>Velocity</td>
<td>No minimum</td>
<td>During WQF: 1.0 ft/sec (See Note 4)</td>
</tr>
<tr>
<td></td>
<td></td>
<td>During HDM flow: 4.0 ft/sec (HDM Table 873.3E)</td>
</tr>
<tr>
<td></td>
<td></td>
<td>(See Note 5)</td>
</tr>
<tr>
<td>Interrelationship Formula for HRT, depth, and velocity</td>
<td>1300 sec²/ft²</td>
<td>No maximum</td>
</tr>
<tr>
<td>Manning's n value</td>
<td>During WQF: 0.20 to 0.30 but 0.24 recommended</td>
<td></td>
</tr>
<tr>
<td></td>
<td>During HDM flow: 0.05 (HDM Table 864.3A)</td>
<td></td>
</tr>
<tr>
<td>Hydraulic conductivity of the soils</td>
<td>There is no minimum set of this parameter at this time set for water treatment purposes.</td>
<td></td>
</tr>
<tr>
<td>in the Biofiltration Swale</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Notes:
1. Bioswale should be designed based on both the WQF and peak flow of the HDM design storm, unless bypass of the larger flows are made.
2. Consult with District Maintenance for whatever bottom width is proposed. If the smallest mower on hand in the District is 4-feet wide, that is the minimum bottom width.
3. From HDM Topic 863: “For large flows, consideration should be given to using a minimum bottom width of 12 feet for construction and maintenance purposes, but depths of flow less than one foot are not recommended.” However, smaller bottom widths are preferred for water quality purposes, in order to limit the tendency at low flows to concentrate into smaller rivulets.
4. Maximum value may be limited if HRT less than 10 minutes, using the Interrelationship Formula. Higher if protected from erosion.
2.2. Restrictions

Successful implementation and utilization of a Bioswale as a Treatment BMP will require proper siting of the Bioswale by coordinating with District Hydraulics and District Landscape Architect. Therefore, it is important to take note of siting restrictions when designing the Bioswale. Bioswales should not be designated as a Treatment BMP if design is outside the range of values presented in Table 2-1, without the approval of the District Design NPDES Coordinator.

- Bioswales in arid regions will require installation of a temporary (or permanent) irrigation system to ensure approximately 70 percent vegetative coverage, and/or must be planted with vegetation that will go dormant outside of the rainy season; consult with the District Landscape Architect to verify that water is available for irrigation. Also assess feasibility. Refer to Section 2.4.2.1 of the PPDG. A permanent irrigation system for a Bioswale should only be recommended if the planting that requires it is specified for aesthetic reasons as part of a landscape plan and there are other irrigated elements in the plan.

- In extremely arid places where permanent irrigation would be required, Bioswales would not be recommended.

- Consult with District Design NPDES Coordinator if Bioswales are proposed at locations having contaminated soils or above contaminated groundwater plumes.

- Bioswales can be used to effectively attenuate some flows depending on the site conditions and site hydrology. Amended soils and vegetation seeding, with correctly designed Bioswales, help in the attenuation of flows.

- Bioswales are not generally subject to setback restrictions as an Infiltration Device is; however, if unusual geotechnical conditions exist, or if a Bioswale is proposed above a retaining wall and the soils are known to be especially erodible or permeable, consult with Geotechnical Design.
3. Getting Started
This section presents the process and parameters incorporated into the design of a Bioswale. Before selecting, sizing, and laying out the Biofiltration Swale, existing site conditions are evaluated to obtain and assess the necessary design parameters that will be used to determine if a Bioswale is applicable and later in the design process. It is assumed that the proposed site has already met the requirements in PPDG Checklists T-1, Parts 1 and 2, and that PPDG Appendix B-2 has been reviewed.

3.1. Preliminary Design Parameters

3.1.1. Rainfall Characteristics
Obtain site Water Quality Flow information for the Bioswale; other hydrologic data such as the event intensity, duration, and frequency should also be obtained for the design of the Bioswale when it is used for the conveyance of the larger events from the tributary area.

A. Flow in the Bioswale under the WQF intensity: Q_{WQF}
The Biofiltration Swale is a flow-based Treatment BMP that is designed to convey and treat the runoff during WQF intensity events, as long as the flow depth, velocity, HRT, and the Interrelationship Formula all met.

The WQF intensity for the project area may be found in the PPDG, Section 2.4.2.2, or by referring to the Basin Sizer program that has been created for Caltrans use, at this link: http://stormwater.water-programs.com/BasinSizer/Basinsizer.htm

The Rational Formula should be used to calculate the runoff entering the bioswale, as shown below:

\[ Q_{WQF} = C \times I \times A \]  

Where
\( Q_{WQF} \) = Water Quality Flow rate (cfs)
\( C \) = runoff coefficient
\( I \) = WQF rainfall intensity (in/hr)
\( A \) = tributary area to the Bioswale (acres)

B. Flow in the Bioswale during the Design Event: Q
The Bioswale must be designed to convey larger during rainfall intensities greater than the WQF, and in fact must handle the peak drainage from the roadway unless an upstream

\[ Q_{WQF} = 0.28 \times C \times I \times A \]  

In metric units: \( Q_{WQF} \) is m³/sec, \( C \) as above, intensity I as mm/hr, and area A as square kilometers.
bypass for the larger events is provided. Absent such diversion, the “Design Event” for the Bioswale must be consistent with the intensity, duration, and frequency of the rainfall event used in the roadway drainage design for that tributary area contributing runoff to the Treatment BMP, as discussed in HDM Topic 831. Additionally, the placement of the Bioswale must not impede the effective drainage of the roadway upstream. Lastly, if the Bioswale is placed with a curve rather than a linear alignment, and is carrying the Design Event, consult HDM Topic 866, Freeboard Considerations.

Note that, notwithstanding the above, there is no overflow release device from the Bioswale, as must be considered for volume-based Treatment BMPs.

3.2. Preliminary Calculations

In order to utilize the Bioswale, the following calculations/analyses need to be performed:

A. Flow depths and velocities at WQF and during Design Event

The flow depth during WQF and the Design Event can be calculated using Manning’s Equation, as shown below. Refer to Figure 3-1 for an illustration of the variables used in the equation.

\[
Q = \left(\frac{1.486}{n}\right) \times A \times R^{2/3} \times S^{1/2}
\]

where

- \( Q \) = flow at defined event (\( Q_{WQF} \) or \( Q_{25} \))
- \( n \) = Manning’s coefficient; recommend using “\( n \)” = 0.24 for \( Q_{WQF} \) and 0.05 for the Design Event \( Q_{25} \)
- \( A \) = Cross-sectional area of the flow in the channel
- \( R \) = Hydraulic Radius = “\( A \)” / Wetted Perimeter (“\( P \)”)
- \( S \) = longitudinal slope, length drop per unit length run

![Figure 3-1 Schematic for Manning’s Equation terms](image)

2 For convenience in this document, this flow will be referred to as \( Q_{25} \), although other recurrence intervals might have been used, as described in HDM 830, Roadway Drainage; confer with District Hydraulics.

3 The depth of flow is shown in various sources as “\( y \)” or “\( d \)”, but with no difference in meaning.
While \( Q, n, \) and \( S \) are known values in the equation, based on the project’s specifics, the area and wetted perimeter (and therefore the hydraulic radius) are unknowns. For this reason, it is easier to set up as a spreadsheet and assign various depths, which allows the area, the wetted perimeter, and the hydraulic radius to be calculated. From these values \( Q \) can be calculated for each of the depths as illustrated on Table 3-1. The row that represents the site is the row that has the nearest \( Q \) that has been calculated to enter the Bioswale during WQF events, from which the depth and velocity can be read. Various hydraulic references present such parametric tables for trapezoidal cross sections.

For example, with the site parameters shown in the upper row (note that Manning’s \( n \) is for the WQF conditions), and with the \( Q_{WQF} \) as 0.51 cfs, the depth in the Bioswale would be approximately 2.4 inches, and the velocity would be 0.23 ft/sec.

<table>
<thead>
<tr>
<th>depth (inches)</th>
<th>( y (=\text{Depth}) ) (ft)</th>
<th>( A ) (ft(^2))</th>
<th>( P ) (feet)</th>
<th>( R ) (feet)</th>
<th>( Q_{WQF} ) (ft(^3)/sec)</th>
<th>Velocity (ft/s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.20</td>
<td>0.100</td>
<td>1.040</td>
<td>10.82</td>
<td>0.096</td>
<td>0.157</td>
<td>0.15</td>
</tr>
<tr>
<td>1.80</td>
<td>0.150</td>
<td>1.590</td>
<td>11.24</td>
<td>0.141</td>
<td>0.311</td>
<td>0.20</td>
</tr>
<tr>
<td>2.40</td>
<td>0.200</td>
<td>2.160</td>
<td>11.65</td>
<td>0.185</td>
<td>0.507</td>
<td>0.23</td>
</tr>
<tr>
<td>3.00</td>
<td>0.250</td>
<td>2.750</td>
<td>12.06</td>
<td>0.228</td>
<td>0.740</td>
<td>0.27</td>
</tr>
<tr>
<td>3.60</td>
<td>0.300</td>
<td>3.360</td>
<td>12.47</td>
<td>0.269</td>
<td>1.011</td>
<td>0.30</td>
</tr>
<tr>
<td>4.20</td>
<td>0.350</td>
<td>3.990</td>
<td>12.89</td>
<td>0.310</td>
<td>1.317</td>
<td>0.33</td>
</tr>
<tr>
<td>4.80</td>
<td>0.400</td>
<td>4.640</td>
<td>13.30</td>
<td>0.349</td>
<td>1.659</td>
<td>0.36</td>
</tr>
<tr>
<td>5.40</td>
<td>0.450</td>
<td>5.310</td>
<td>13.71</td>
<td>0.387</td>
<td>2.035</td>
<td>0.38</td>
</tr>
<tr>
<td>6.00</td>
<td>0.500</td>
<td>6.000</td>
<td>14.12</td>
<td>0.425</td>
<td>2.446</td>
<td>0.41</td>
</tr>
</tbody>
</table>

Table 3-1: Manning’s Equation solved for particular conditions by assuming depth values

The same spreadsheet or parametric table would be used when analyzing the flow during the Design Event, except that the \( Q_{25} \) and Manning’s \( n \) as 0.05 would be used. Note: The higher of the two flow depths calculated should be used to set the constructed depth of the Bioswale, including any freeboard that may be required.

The geometric parameters set during this step, the base width, side slope ratio, and longitudinal slope, are accepted at the trial values, pending the check for Hydraulic Residence Time and the Interrelationship Formula.

### B. Hydraulic Residence Time

The minimum travel time within the Bioswale, termed the Hydraulic Residence Time (HRT) is 5 minutes. This can be checked after the proposed Bioswale site is analyzed using Manning’s Equation, as discussed above. The velocity associated with the \( Q_{WQF} \) is determined, and the HRT calculated using the proposed length of the Bioswale:

---

4 A downloadable spreadsheet can be found on the OSWMD intranet website
HRT = L / (60 x VWQF)

where
L = proposed length of the Bioswale (ft)
HRT = Hydraulic Residence Time (minutes)
V_WQF = velocity at Q_WQF (ft/sec)
60 = conversion from seconds to minutes

If the HRT is less than 5 minutes, then the length of the Bioswale should be increased, or the velocity at Q_WQF should be decreased by increasing the width of the Bioswale or by decreasing the slope.

C. Interrelationship Formula during WQF
Upon determining that the HRT, d_WQF, and V_WQF meet their respective design criteria, the Interrelationship Formula shown below also must be satisfied, as the maximum allowed depth of flow and velocity may be restricted if the HRT is less than 5 minutes.

\[
\frac{(HRT \times 60)}{(d_{WQF} \times V_{WQF})} \geq C
\]

where:
HRT = Hydraulic Residence Time during Q_WQF (minutes)
60 = conversion factor from minutes to seconds
d_WQF = depth of flow at Q_WQF (ft)
V_WQF = velocity of flow at Q_WQF (fps)
C = constant: 1,300 (sec^2/ft^2)

3.3. Other Comments
1 The Bioswale should be designed with the maximum length (in direction of flow) as allowed by the site. In general, the flatter the slope, the shorter the Bioswale length required to meet Treatment BMP requirements.

2 The width of the Bioswale is often the most easily changed site variable if the original proposed dimensions do not satisfy depth, velocity and Hydraulic Residence Time (HRT) criteria at WQF, but sometimes the slope may be reduced.

3 The tributary area upstream of Bioswales is usually not as large as the tributary areas for volume-based Treatment BMPs.

4 Calculations for the Bioswale, especially the HRT, are easier if most or all of the Q_WQF enters at a discrete location at the upstream, rather than at distributed locations along the length of the Bioswale. However, if the flow enters the Biofiltration Swale continuously along the length of the swale or at multiple discrete locations, other rational methods should be employed. In the case of
continuous flow entering the swale, the PE may wish to initially calculate the depths and velocities at selected points along the swale to verify that the depth or velocity has not exceeded the maximum allowed values. This same calculation could also be used if there is a change of grade. The length of the swale that would qualify as a Biofiltration Treatment BMP must be upstream of the location where either the maximum depth or velocity was exceeded. The calculation of the HRT when the WQF enters at multiple (actual entry points or discretized from continuous flow) entry points could be done by calculating the HRT for the flow from each of the discrete entry points, and then taking a weighted average of the HRTs for the entire flow over the length that qualifies as a Biofiltration Treatment BMP; velocity and depth criteria would still need to be met.

5 To provide adequate hydraulic function, a swale should also be sized as a conveyance system calculated according to criteria and procedures for conveyance of design storm flows and scour associated with the peak drainage facility design event.

6 Check dams within the Bioswale may be considered if the HDM criteria are met, but the HRT, velocity, or length requirements are not met due to the steepness of the proposed Bioswale. Follow the guidance for Temporary Check Dams located in the Caltrans Construction Site BMP Manual. Additionally, check dams should be constructed of soil, placed a maximum of 20 ft apart, have 4H:1V slopes, be a maximum height of 9 inches and be vegetated. Placement should not impede the flow of the HDM Design Event. Check dams must be maintained.

7 Fiber roll check dams within the Bioswale may be considered. An Interim Guidance on Permanent Fiber Roll Check Dam is available. Consultants should contact OSWMD. Caltrans employees can download the guidance on the Design HQ intranet site at http://onramp.dot.ca.gov/hq/design/stormwater/guidance/FRCD_Guidance_040110.zip.

Permanent fiber roll check dams are sometimes specified as an erosion control measure. The dams are used in final channels with non-erosive flows to slow the water and create a series of micro-pools to promote vegetative growth. However, the check dams have the potential to create erosion if the permitted flow velocities are exceeded or scour prevention is not installed. While check dams can be beneficial in promoting vegetation and minimizing erosion, their use must coordinated with Maintenance. Mowing or grading will interfere with the dam operation. Consult your project Landscape Architect or Design Stormwater Coordinator for more information.

8 If the criteria for the Bioswale cannot be met, then several options are available: i) the proposed swale could be considered as a Design Pollution Prevention (DPP) BMP – Concentrated Conveyance System rather than as a Treatment BMP;
ii) an alternative Treatment BMP could be considered at that location; iii) the RWQCB could be petitioned for reduced WQF intensity (see PPDG Section 2.4.2.2) if the Bioswale criteria could be met at a lower $Q_{WQF}$. 
4. BMP Layout and Design

4.1. Layout

4.1.1. Location
Biofiltration Swales, and the related Biofiltration Strips, are probably the least expensive Treatment BMPs for an area, if the proposed location is suitable. They can be placed as a stand-alone device in order to meet MEP treatment requirements, or can be placed usually as the first BMP encountered in a “treatment train” and should be considered upstream of other Treatment BMPs that benefit from pretreatment to reduce sediment loading, such as Infiltration Devices, Detention Devices, and Wet Basins. However, to provide effective treatment of runoff, the proposed location must be able to support the chosen vegetation; locations should be sought that have sufficient open space, adequate sunlight for vegetation growth, and topography to meet the hydraulic requirements.

Entry of runoff into a Bioswale may enter as sheet flow along its length, and/or from a concentrated conveyance. If the latter, it may require energy dissipation to prevent erosion. One location that should receive special consideration is at the end of a bridge structure. Refer to Section 4.2.2.

4.1.2. Maintenance
Bioswales need sufficient space for various maintenance activities, including enough space for maintenance vehicles and during cleaning, repair, or inspections. District Maintenance should concur with the proposed Bioswale location.

4.2. Site Specific Design Elements

4.2.1. Turf Reinforcement Mat (TRM)
When the flow velocity exceeds 4 ft/sec for the design storm, a geotextile, such as turf reinforcement mat (TRM), may be used to prevent scour within the swale. The use of a TRM within the swale for velocities higher than 4 ft/sec during design storm events does not negate the need to meet all the design criteria during Water Quality events as presented in section 2.2. A nonstandard special provision for use of a TRM in a Bioswale has been developed and is available for use.

If the flow characteristics do not require a TRM, a temporary erosion control blanket or RECP (Rolled Erosion Control Product) may still be needed to protect the soil from concentrated flow that may occur the first winter before vegetation can be established. Hydroseeding is not recommended for areas that will receive concentrated flows.
4.2.2. Inflow Detailing
Consider detailing such as flared end sections where concentrated flow enters a Bioswale to prevent erosion.
5. PS&E Preparation

This section provides guidance for incorporating Bioswales into the PS&E package, discusses the typical specifications, and presents information about estimating the construction costs.

5.1. PS&E Drawings

Bioswale Treatment BMPs do not have standard drawings for the device as a complete feature, but there are several sheets that could be placed in the PS&E package. The PS&E drawings for most projects having Bioswales will include:

1. **Layout(s):** Show location(s) of the Bioswales; this is a recommended option, as its use will help verify to the RWQCB or to a third party that this Treatment BMP was placed.

2. **Contour Grading(s):** As Bioswales are primarily earthwork features they should be shown on Contour Grading sheets. Any other associated grading surrounding the Bioswale should be shown on these sheet(s).

3. **Construction Details:** Inflow and outflow detailing should be shown (if, for example, a lined channel is at the upstream or downstream end of the Bioswale). These sheets may be shown in the Drainage Detail sheets, at the option of the Project Engineer.

   Final grade should maintain sheet flow across the top hinge point of a Bioswale (e.g. when Bioswale is adjacent to a roadway shoulder). To avoid mulch and plant material from blocking the sheet flow into the Bioswale, a construction detail showing the final grade including mulch may be required.

4. **Drainage Plan(s), Profiles, Details, and Quantities:**

   a) Drainage Plan sheets should show each Bioswale in plan view, along with other existing (or proposed) drainage conveyance devices that direct the runoff into the device.

   b) Drainage Profile sheets should show the Bioswale in profile within the drainage conveyance system. These sheets should also call out the specific Bioswale inlet and outlet flow line (surface) elevations.

   c) Drainage Detail sheets should show any other structural detailing needed for the construction of the Bioswale not provided elsewhere in the contract plans, but see also the comments for the Landscape Plans.
d) Drainage Quantity sheets should include all pay and non-pay items associated with the construction of the Bioswales, except for those items that will be placed on the Summary of Quantities sheets.

5 **Landscape Plans and/or Erosion Control Plan:** these items should be prepared for the interior of the Bioswale and nearby areas, for soil amendments, planting, and similar items (prepared by the District Landscape Architect).

6 **Other Sheets:** Water Pollution Control, Construction Staging, Utility Plans, Irrigation Plans (if permanent irrigation will be placed, or the existing system modified), and other sheets should be considered as appropriate for the construction of Bioswales on a project-specific basis.

If Bioswales will be constructed at multiple locations, a “Locations of Construction” table could be considered. This table could present the stationing and other location information could be considered. This table may be incorporated into an existing drawing if there is room (such as a Title, Layout or Construction Detail), or may be developed as a separate drawing if necessary.

While every effort has been made to provide accurate information here, the Project Engineer is responsible for incorporating all design aspects of Bioswales into the PS&E in accordance with the requirements of Section 5 of the Department’s Plans, Specifications And Estimates Guide.

### 5.2. **SPECIFICATIONS**

Contract specifications for Bioswale Treatment BMP projects will include Standard Specifications, Standard Special Provisions (SSPs), and non-Standard Special Provisions (nSSPs).\(^5\) Where indicated below, specific nSSPs have been developed by the HQ Office of Storm Water Management.

The special provisions for the various items of work directly needed to construct the Bioswale could be organized under the blanket heading of ‘Bioswale’ (with some or all of these items listed as subheadings, and payment would be made for by ‘each’ Bioswale), or separate listings could be made for each contract item of work (with separate measurement and payment). The Project Engineer and the District Office Engineer should consider which method would better serve the particular project.

#### 5.2.1. **Standard Special Provisions**

Listed below are SSPs that would typically be used for a project that constructs a Bioswale Treatment BMP; the Project Engineer should consider the construction of Bioswales in the

---

\(^5\) Standard Specifications will not be included but merely referenced in the contract’s special provisions.
context of the entire project to determine if other SSPs may be required.

1. Order of Work6 (SSP 05-010)
2. Water Pollution Control (SSP 07-340 or 07-345)
3. Clearing and Grubbing (SSPs 16-050, or 16-060, or 16-070)
4. Earthwork (SSP 19-010; and/or Miscellaneous Earthwork Clauses, SSP 19-040; and/or Measurement And Payment (Earthwork), SSP 19-590)
5. Erosion Control (Erosion Control (Type C), SSP 20-030; or Erosion Control (Type D), SSP 20-040)
6. Temporary Irrigation (as Plant Establishment Work, SSP 20-550)
7. Highway Planting (SSP 20-350)
8. Slope Protection (SSP 72-010)

5.2.2. Non-Standard Special Provisions

Typical placement of BioSwales will not require any nSSPs. However, the Project Engineer and the District Landscape Architect must ensure that non-Standard Special Provisions, if needed, have been developed based on site-specific requirements. HQ Landscape will most likely be the approval group for any Bioswale nSSPs developed.

5.3. Project Cost Estimates

Project Cost Estimates (PCEs) are required at every phase of the project, as discussed below. Section 7 of the Ready-to-List and Construction Contract Award Guide (RTL Guide) should be consulted for more details on preparing construction cost estimates.

5.3.1. PID and PA/ED Phases

At the PID phase of the project, the construction cost could be estimated based on the findings of the BMP Retrofit Pilot Program Final Report, which was $21/ft³ of WQV treated, exclusive of right of way.7

To determine an initial cost estimate using this value simply use the following equation:

Initial construction cost = ($21/ft³ x run-up factor) x tributary area x 0.75/12

This estimate will need to be modified as the project progresses. If some design is conducted during the PA/Ed phase of the project, it is possible that a more refined estimate

---

6 The Order of Work should indicate that no runoff from construction should be allowed to flow into the BioSwale until all upstream areas contributing runoff are stabilized and BioSwale vegetation established.

7 In 1999 dollars; contact District Office Engineer for appropriate run-up factors based on local experience. Note that costs were given in units more appropriate for a volume-based Treatment BMP, for the comparison purposes within that report. The formula shown has been modified slightly to account for the otherwise incorrect units, and should only be used for estimating purposes in these phases of the project.
could be made using the methods in Section 5.3.2. A cost escalation should be added for projects that are anticipated to advertise more than a year after the date of the estimate.

5.3.2. **PS&E Phase**

As the design process proceeds into the PS&E phase, unit quantities would be available, and the associated units costs would need to be obtained, as accurate costs are critical. When developing costs based on unit quantities, the cost estimates should be based upon the Department’s *Contract Cost Data Book* or recent, similar projects in the District, as appropriate. The Project Engineer should use SSPs and nSSPs from Sections 5.2.1 and 5.2.2 of this document, as well as those added on a project-specific basis, to develop a list of items for which item costs should be developed. The Project Engineer should carefully check that all items of work are accounted for either as pay or non-pay items.
6. References


4. WERF. 2004. Post-Project Monitoring of BMPs/SUDS to determine Performance and Whole Life Costs


7. King County 2005. *Surface Water Design Manual*

8. California Department of Transportation (Caltrans). *Storm Water Quality Handbooks: Wet Basins Design Guidance* handbook (under development at time of printing)

9. California Department of Transportation (Caltrans). *Storm Water Quality Handbooks: Supplemental Design Guidance* handbook (under development at time of printing)


A Biofiltration Guidance-Vegetation

A.1 Overview
This Appendix provides design guidance for the vegetative cover component of Biofiltration BMPs (i.e., bioswales and biostrips). For Biofiltration BMPs, District Landscape Architecture is responsible for selecting vegetation, and developing the appropriate plans, specifications, details, and estimates for contract documents. Incorporation of other improvements such as soil amendment and irrigation may be required to satisfy site conditions. For Biofiltration BMPs to be effective, it is necessary to establish a healthy, sustainable vegetative cover.

The District Landscape Architect or delegated representative is to verify that specific project requirements, necessary to provide vegetative cover for the Biofiltration BMPs, are documented in the Storm Water Data Report (SWDR) and prepared for PS&E.

A.2 Soil Testing & Investigation
It is important to identify roadside soil characteristics of a site to determine its erosion potential, infiltration qualities, and ability to establish vegetative cover.

When analyzing soils information for Biofiltration BMPs the soils test should report the following:

- USDA-NRCS particle size distribution (soil texture) – ASTM D6913-04e1
- Organic matter percentage – ASTM D2974-07a
- Infiltration rate –ASTM D3385-03
- pH
- Salinity
- Sodium (Na)
- Boron (B)
- Nitrogen (N)
- Phosphorus (P)
- Potassium (K)

1) Information on soils should be requested as part of the geotechnical investigation. Soils characteristics such as USDA Soil Group, USDA-NRCS particle distribution (soil texture), organic matter percentage, and infiltration rate should be requested. If the Geotechnical Report has been completed without soils information, or if no Geotech Report is prepared for the project, then the Landscape Architect should collect soil samples and submit them to a soils testing lab for processing.
2) If soils information cannot be obtained for the project site, then soils information from a reference site should be evaluated. To be valid, the reference site must have characteristics similar to the project site such as slope aspect (north-facing, etc.) topography (cut or fill slopes), climate, geology, and existing vegetation.

A.3 Soil and Planting Area Preparation

Roadway construction that requires excavation or embankment work typically results in soils unable to support desirable vegetation. Grading operations, including compaction, produce soils of pulverized parent material with inverted soil horizons that lack nutrients, organic matter, rooting depth, and pore space. When these site conditions are anticipated, an assessment should be made by District Landscape Architecture to determine the level of treatment necessary to support vegetation. Coordination with the project engineer is recommended to ensure harvesting of suitable roadside soils within the project limits is considered.

Compost and organic slow release fertilizers provide a better method to provide nutrients for desirable plant species than conventional commercial fertilizer. Because commercial fertilizers provide nutrients that are readily available, the germination and growth of invasive weeds is encouraged rather than desirable slower growing species. Incorporating compost and applying organic slow-release fertilizer releases nutrients gradually, thereby promoting the germination and establishment of perennial grasses and woody shrubs. In addition to providing long-term nutritional benefits as a soil amendment, compost can be applied as a 1” to 2” mulch layer on biostrips to prevent the germination of most invasive annual weed species. When a compost mulch layer is specified, apply the seed on top of the compost to ensure that desirable seed species will germinate.

The following should be considered when preparing soil for biofiltration BMPs:

1) The soils in areas designated for biofiltration should be ripped and cultivated to a minimum depth of 12-inches to relieve surface compaction. For slopes steeper than 4:1(H:V), coordinate with the geotechnical engineer and District NPDES Coordinator. Deeper ripping may provide long term treatment benefits by increasing infiltration and water holding capacity, especially where soils are shallow.

2) Compost can be used to boost soil nitrogen and organic matter percentage if a soils report indicates a deficiency for these. A minimum of 1100 lb total N/acre distributed throughout the top 12 inches of soil is the end goal. For poor soils, compost should be incorporated at a minimum rate of 400 cuyd/acre to a minimum depth of 12-inches to help restore soil organics, rooting depth, porosity and nutrients (carbon and nitrogen). Compost should be specified as per SSP 20-056, “Compost, Incorporate”.

3) Harvest topsoil from areas to be graded. Compost use may not be required if topsoil has an acceptable level of organic matter. Designate topsoil harvest and stockpile locations on the plans. Plant harvested topsoil with a temporary
vegetative cover of appropriate native species to maintain microbial soil health. Include details for re-application and placement of topsoil. Coordination with the project engineer is recommended in ensuring staging requirements and environmental issues are considered.

4) Where invasive weeds are pervasive, specify that weed seeds be germinated and resulting weeds killed or include mulch for weed control. Germination of an existing seed bank is a very effective method for controlling weeds. Planting by hydroseeding or other methods should be done after one or more weed germination cycles. Weed germination is only practical when temporary irrigation is employed. A 1”-2” layer of compost mulch is also an effective method to prevent germination and competition from annual weeds. Keep mulch clear of flow lines for bioswales.

5) Incorporate amended soil as part of slope construction when specifying Mechanically Stabilized Embankments (MSE) for steep embankments. Placing amended soil at the proposed slope face can be done as part of placing each geotextile reinforced lift. An open weave erosion control blanket should be used to contain the material at the slope face by using a “wrap-back” method. The slope can be hydroseeded upon completion of the MSE.

**A.4 Irrigation Strategies**

California’s diverse microclimates make establishing vegetation difficult and sometimes impossible without supplemental irrigation. Harsh site conditions often limit the ability of the plantings to survive and thrive to the extent necessary for successful treatment. Under these conditions, irrigation should be provided when connecting to a water source is practicable.

The following criteria should be considered when designing biofiltration BMPs:

1) If vegetative cover goals (minimum 70%) are not achievable under normal rainfall, a temporary irrigation system will be required until plants are established.

2) If establishment of vegetative cover is required prior to normal rainfall, or the anticipated contract schedule necessitates germination prior to winter, then the use of a temporary irrigation system to establish the vegetation prior to the rainy season is recommended.

3) If the proposed biofiltration BMP is adjacent to or within an irrigated landscape then utilization of the existing water service for permanent irrigation is recommended to ensure design compatibility with existing highway planting.

**A.5 Planting Strategies**

The following criteria should be used as a general measure of successful Biofiltration BMP installation:

1) Within the first year, a minimum of 70 percent vegetative cover is achieved.
2) The Biofiltration BMP does not exhibit rills, gullies, or visible erosion that is contributing to the export of sediment.

3) Biofiltration BMPs located within a landscaped urban interchange should be designed to complement the adjacent existing highway planting.

California’s diverse microclimates require equally diverse strategies for successfully establishing vegetation. The following planting strategies have been successfully used for roadside revegetation and should be considered for biofiltration BMPs where appropriate:

1) Provide temporary cover until the desired percent cover of vegetation is achieved. For challenging sites, it may take more then one growing season to establish adequate vegetative cover. This is especially true for shrubs and woody perennials on harsh sites. Temporary cover is usually provided through the use of short-term, degradable erosion control products such as rolled erosion control products (RECPs), wood chips and compost, straw, and hydromulchs. These products vary in how long they will last. For example, straw can be expected to last through a single rainy season while a high quality coconut fiber erosion control blanket will usually function for 3 years.

2) Strive for cost effective solutions. In most cases, the temporary cover product with the greatest longevity will also be the most expensive. While plant performance, slope steepness, slope inclination, slope aspect, and soil characteristics must be considered, avoid over-design. Specify materials as warranted by specific project conditions. For example, a cost-effective project design may use straw and hydrosed on areas of good soil and gentle slopes, reserving compost and coir netting for steep, cut slopes.

3) Specify drought tolerant grasses, with a goal to use as many appropriate local native species as possible. Species that become dormant during the dry season are acceptable as long as biofiltration performance is not affected. In Southern California, it is preferable to select plant species that are not dormant during the winter and spring (wet season).

4) Combine hydroseeding and direct planting. Some plant species favor particular planting methods, so special provisions may need to be made if these species are to be used. Many plant species can be applied by hydroseeding. Other plants are better established as liner, container, or plug plant material and can be installed in previously seeded areas, following germination. This method can be effective for bioswales when the upland zone on the banks is hydroseeded and the hydrophilic zone in the bed is planted with sedge, grass, and rush liners.

5) Specify erosion control blankets or other RECPs in areas that will receive concentrated flow. Although hydroseeding may be appropriate for planting portions of bioswales, it should not be used in locations that will receive concentrated runoff. Liner, container or plug plant material is a better choice in these areas.
A.6 Restrictions for Plant Selection

Nearly half of the bulk solids collected in the structural treatment BMPs consists of plant litter such as leaves and twigs. However, trees and shrubs provide both functional and aesthetic benefits to highway roadsides. To limit maintenance requirements of downstream treatment BMPs and especially basins and trash BMPs such as Gross Solid Removal Devices (GSRDs), trees and shrubs selected for banks of biofiltration swales should not contribute large amounts of plant litter (e.g., bark, leaves, flowers or seeds) to the BMP.

A.7 Drainage Facilities

Accommodations for drainage facilities upstream and downstream from biofiltration BMPs should be considered as follows:

1) Provide a concrete apron large enough to prevent overgrowth and reduce clogging:
   a) around drainage inlets located in bioswales,
   b) at culvert inlet and outlets such as flared end sections.
   This enhances performance and facilitates inspection and maintenance. Ensure apron design meets engineering standards as per Highway Design Manual.

2) Include armoring or other forms of scour protection such as the use of rock or turf reinforcement at the transitions from piped or other hard surface conveyance to vegetated areas.

3) Avoid standing water or rock placement on poorly draining soils in order to prevent vector (mosquito) breeding.

A.8 Plant Establishment Period (PEP)

PEP ensures project success by maintaining plants during a period when mortality rates tend to be high. This is true for Highway Planting, as well as for revegetation planting that includes grasses and forbs, and especially native grasses.

The following should be considered when requiring PEP for biofiltration BMPs:

1) Biofiltration BMPs that are graded, constructed and planted as part of a roadway construction contract should have a 1-year PEP. Depending upon the type of construction and order of work, the PEP may run concurrently with other work.

2) Work to be performed during the PEP should include the following when applicable for the project:
   a) Weed control and removing inappropriate plant species (as field identified by a qualified specialist),
   b) Watering plants as necessary,
   c) Mowing and other vegetation management,
   d) Repairing rills, gullies, and other damage caused by erosion and scour,
   e) Reseeding bare or repaired areas,
Appendix A

Biofiltration Guidance-Vegetation

f) Replacing liner, container, or plug planting,
g) Monitoring and repairing irrigation system (permanent or temporary),
h) Removing accumulated sediment and debris, and
i) Removing temporary irrigation system at end of PEP.

3) The following activities may be performed during the PEP and done as extra work:
   a) Additional planting,
   b) Additional irrigation, and
   c) Monitoring plant growth and establishment.

4) Ideally, a 3-year contract to perform plant establishment work should follow immediately after completion of the roadway contract that installed the Biofiltration BMPs. If practical, the follow-up contract may include the Biofiltration BMPs of several construction projects in proximity of each other.

A.9 Definitions:

1) Vegetative Cover: (This definition is paraphrased from the draft NPDES Construction General Permit) Actively growing plant matter that is uniform, self-sustaining (perennial) and in contact with the soil. Live perennial vegetation may include grasses, grass like species, forbs, and some broad leaf species that are ground covers, low shrubs, or a combination. The area shall have a minimum vegetative cover of 70% within the first growing season. The remaining 30% shall be covered by fallen plant litter, standing dead plant material, or mulch.

2) Temporary Cover: Temporary cover consist of erosion control materials that provide short-term, interim protection of disturbed soil areas. Temporary cover products include erosion control blankets and other RECPs, straw, composts and mulches, and hydromulches. These products are generally composed of biodegradable and photodegradable materials with variable longevity.

A.10 Additional Resources

See locations below for additional information regarding planting methods and strategies for slope stabilization:

- Several studies on biofiltration BMPs, erosion control, and revegetation can be found on the Caltrans Department of Environmental Analysis webpage at: http://www.dot.ca.gov/hq/env/stormwater/special/newsetup/index.htm.

- The Caltrans Revised Universal Soil Loss Equation (RUSLE2) and the Erosion Prediction Procedure (EPP) provide a software tool and guidance for predicting surface erosion and selecting BMPs. The EPP appendixes include information on collecting and processing soil samples. (Under development)
- The Erosion Control Technology Council (ECTC) is a non-profit organization dedicated to developing performance standards, uniform testing procedures, and guidance on the application and installation of rolled erosion control products (RECPs). Information for selection and installation of RECPs based upon longevity, slope inclination, C-factor, and shear strength is given for various classes of products. Access their webpage at: [http://www.ectc.org/](http://www.ectc.org/).

- The Soil Resource Evaluation (SRE) provides a stepwise process for regeneration and re-vegetation of drastically disturbed soils. Information on plant rooting depth, plant available water, soil nutrient levels, etc. can be found here. This information can be found on the HQ Landscape Architecture Program webpage at: [http://www.dot.ca.gov/hq/LandArch/research/index.htm](http://www.dot.ca.gov/hq/LandArch/research/index.htm).